

1           **Reef flat flow dynamics for a nearly closed fringing**  
2                           **reef-lagoon: Ofu, American Samoa**

3           **Samantha A. Maticka<sup>1,2</sup>, Justin.S. Rogers<sup>1</sup>, C.Brock Woodson<sup>3</sup>, Ben Hefner<sup>3</sup>,**  
4                           **and Stephen G. Monismith<sup>1</sup>**

5           <sup>1</sup>The Bob and Norma Street Environmental Fluid Mechanics Laboratory, Department of Civil and  
6                           Environmental Engineering, Stanford University, Stanford, CA, 94305-4020, USA

7           <sup>2</sup>Geosciences Montpellier, Université Montpellier, CNRS, Univ Antilles, Montpellier, France

8           <sup>3</sup>College of Engineering, University of Georgia, Athens, GA, 30602, USA

9           **Key Points:**

- 10           • Wave driven flows and flow dynamics vary with tidal water level  
11           • Coral reef lagoon appears closed at high tide and open at low tide  
12           • Flow structure for wave-driven flow resembles surf-zone when the lagoon is closed,  
13           and can be modeled using a wave-based eddy viscosity

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Corresponding author: Stephen Monismith, [monismith@stanford.edu](mailto:monismith@stanford.edu)

## Abstract

We discuss observations of tidally varying wave-forced flows in the reef system on Ofu, American Samoa, a barrier reef and lagoon system that appears open at low tide and closed at high tide. At high tide, the free surface pressure gradient nearly balances the radiation stress gradient in the depth-integrated momentum equation. At depth, there is an imbalance between these two forces, generating an undertow and flows that turn alongshore and, for some of the time, offshore, behavior similar to rip currents observed on beaches. At low tides, the wave forcing drives purely onshore flows. In general, wave transport (including assumed roller behavior) is important to determining the total net transport. In both cases, the vertical structure of this flow can be predicted with some accuracy using the surf-zone model of Svendsen (1984), albeit with an eddy viscosity that is proportional to the rms wave velocity. While the dynamically closed nature of the lagoon mostly suppresses cross-reef transport, there is always some flow through the lagoon, with the strongest flows occurring at high tides and when the wave forcing is strongest.

## Plain Language Summary

Waves and flows observed in the coral reef lagoon system located on the south shore of Ofu, American Samoa show that flows in the lagoon are driven by incident swell modulated by tidal variations in depth on the steep fore reef and on the shallow reef flat. At low tide, flows are across the reef flat from the fore-reef to the lagoon behave as though the lagoon is open to the ocean, whereas at high tide, flows into the lagoon are strongly limited by the resistance felt by the flow out of the lagoon. As a consequence, flows on the reef flat can develop an undertow, as is seen on beaches, although this varies with position on the reef flat. Nonetheless, overall flows in the lagoon are strongest at high tides and weakest at low tides.

## 1 Introduction

The wave-driven flow through fringing reef-lagoon systems is often described using a one dimensional model (Symonds et al., 1995; Coronado et al., 2007; Hearn, 1999; Hench et al., 2008; Lowe et al., 2009; Taebi et al., 2011; Monismith et al., 2013; Zhang et al., 2012; Sous et al., 2020), and has shown to be an important flushing mechanism for many reefs (Munk & Sargent, 1954; Callaghan et al., 2006; Davis et al., 2011; Symonds et al., 1995; Hearn, 1999; Hench et al., 2008; Lowe et al., 2009; Rogers et al., 2017), i.e.,

45 one that is important to determining the response to surface heating (Zhang et al., 2013),  
46 or the extent to which benthic communities can change bio-geochemical properties of la-  
47 goon waters (Kowek et al., 2015). In the 1D model, waves approach from offshore, shoal,  
48 steepen, and break near the reef crest, leading to a setup on the reef-flat that drives flow  
49 into the lagoon, and out through channels in the reef. The area on the fore-reef where  
50 waves break is referred to as the surf zone; the momentum balance there is between the  
51 pressure gradient force due to variations in the free-surface height and the radiation stress  
52 gradient. On the reef flat, the balance is generally assumed to be between the pressure  
53 gradient force and the bottom drag on the reef flat.

54 The strength of the wave-driven flow over the reef flat depends on the slope of the  
55 free-surface, and, thus on setup of the water level in the lagoon (Lowe et al., 2009). In  
56 what follows, a closed lagoon vs. open lagoon refers to how easily the incoming ocean  
57 water can leave a reef system, behavior that is determined by the geometry of channel  
58 openings (Gourlay, 1996). Using 2D simulations with different idealized geometries, Lowe  
59 et al. (2010) found that lagoon setup varied with the width of the outflow channel: Setup  
60 in the lagoon was larger for systems with narrow outflow channels, i.e., systems that were  
61 nearly closed. In this case, the flow was not one dimensional, but instead was similar to  
62 what is seen on beaches, i.e. rip currents as a mechanism for water returning to the ocean  
63 as opposed to flows out channels (Lowe et al., 2010). These simulations also show that  
64 the cross-reef transport is reduced, and can be redirected along the reef. Thus it appears  
65 that the 1D model best fits systems that have relatively large outlet channels, i.e., that  
66 appear to be *open*. These two limits might be exemplified by Kaneohe Bay as a closed  
67 system (e.g. Lowe et al. (2009)) and the reef on the north shore of Moorea as an open  
68 system (e.g. Monismith et al. (2013)).

69 Using theory and numerical models, Lindhart et al. (2021) investigated how the  
70 flow dynamics of an idealized version of the reef-lagoon system found on Ofu, American  
71 Samoa varied tidally. They found that depending on water level, the system could be  
72 considered either open or closed. They suggested that the extent to which reef systems  
73 should be classified as open or closed depends on the momentum balance operating on  
74 the reef flat as opposed to the geometry of the reef lagoon. They define open systems  
75 as ones exhibiting a balance on the reef flat between an onshore, wave-generated pres-  
76 sure gradient balanced by friction, and closed systems as ones for which the onshore ra-  
77 diation stress gradient is opposed by an offshore pressure gradient.

78 In the present paper, we use field observations made on Ofu, American Samoa in  
79 March 2017 to look in detail at the dynamics of flows on the reef flat for times when the  
80 Ofu reef-lagoon system appears open and times when it is closed. We consider below the  
81 relationship between setup and wave forcing (section 3.2), transports on the reef flat and  
82 in the lagoon (section 3.3), the depth and wave-averaged momentum balance (section  
83 4.1), and the vertical structure of flows on the reef flat in light of the surfzone model of  
84 Svendsen (1984) (section 4.2). Rogers et al. (2018) describe the general conditions (tides,  
85 waves, etc.) observed during this experiment but focuses on the connection between the  
86 statistics of reef topography and frictional drag in the lagoon. In the present paper we  
87 focus on the behavior and dynamics of wave-driven flows on the reef flat, especially con-  
88 sidering the extent to which the system is open or closed.

## 89 **2 Field Site and Instrumentation**

90 All of the measurements we report here were made in and near the reef lagoons on  
91 the south shore of Ofu, American Samoa (14.28S, 169.78W) (Fig. 1) from March 10-28,  
92 2017. Ofu is almost entirely surrounded by a fringing reef extending ca. 100-200 m from  
93 the shore. The reef flat itself is about 100 m wide, which is significantly narrower than  
94 many other reef sites (Wiens et al., 1962; Hench et al., 2008; Lowe et al., 2009). The reef  
95 flat has a tidally-averaged depth of  $\sim 0.5$  m and has a fairly uniform coverage of rough-  
96 ness features on the order of 5-20 cm high, few of which are living coral. The associated  
97 lagoon is  $\sim 2$  m deep and has significant coral coverage with features that range from 5  
98 cm to 2 m (Chirayath & Earle, 2016; Oliver & Palumbi, 2009). Physically, Ofu is a com-  
99 mon fringing reef-lagoon system; i.e., the lagoon axis is parallel to shore and has chan-  
100 nels through the reef to connect the lagoon to the ocean. However, compared to systems  
101 studied previously (e.g. (Hearn, 1999; Hench et al., 2008; Lowe et al., 2009; Coronado  
102 et al., 2007)), the Ofu reef system has only a few narrow channels as well as higher fric-  
103 tion in the lagoon due to the large coral structures; thus, it is an excellent site for study-  
104 ing wave-driven flow dynamics of closed lagoons.

105 The field study employed instruments measuring velocities, pressures, and temper-  
106 atures throughout the lagoon to observe spatial and temporal variability in both waves  
107 and mean flows on the fore-reef, in the lagoon and in the exit channel (pass). Figure 1  
108 and Table 1 (see also Maticka (2019)) list the various instruments deployed throughout  
109 our 18 day experiment that will be discussed below, as well as their various sampling pa-

rameters. Beside the instruments shown in Fig. 1, a weather station was also situated on the beach c.a. 1.4 km SW from the field site.

In this paper we focus primarily on the behavior of waves and flows using data from instruments located along the cross-reef D-transect, sites starting with ‘D’, D-3, D-4, D-5, all situated on the reef flat, D0 in the lagoon, and FR5 and FR15 on the fore-reef. Along this transect the reef flat is  $\sim 135m$  wide, and nearly flat with a slight increase in depth toward the lagoon ( $dh/dx \approx 4 \times 10^{-3} m/m$ ). This line included RBR solo pressure loggers (accuracy = 1 cm; precision = 0.2 mm) except at FR15, where there was a Seabird SBE26+ wave/tide recorder (accuracy = 1 mm; precision < 0.4 mm). At stations D0 and FR15, velocity and wave measurements were made with 2Mhz and 1Mhz Nortek Acoustic Doppler Profilers respectively.

Detailed velocity profiles on 3 s intervals were obtained at D-4 using a Teledyne RDI vADCP configured to run autonomously (Hefner et al., 2019). Unfortunately besides the failure of the vADCP after 5 days due to instrument flooding, the RBR pressure sensor deployed at D-4 also failed to record any data. Nonetheless, because of the unique data provided by the vADCP, the analysis below will primarily focus on those 5 days.

Finally, in addition to moored instruments, several releases of shallow GPS drifters constructed with radio-tracked dog collars (Herdman et al., 2015) were conducted in the lagoon for both high and low tide conditions to observe Lagrangian flows in the lagoon.

A key aspect of analyzing the behavior of flows in systems like the Ofu reef is removing wave signals, i.e., performing wave-averaging, which we will denote in all of what follows for any variable,  $f$  by  $\bar{f}$ . In analyzing our data we used three approaches for this averaging:

(1) Half hour averages of water levels: Wave statistics and mean water levels for all the pressure sensors were computed this way so as to match the half-hourly wave burst data acquired by the SBE26+ at station FR15.

(2) One hour averages of data acquired by the vADCP at station D-4. This was done to balance removal of variability associated with both waves and instrument noise with temporal resolution of the flow. This approach also facilitated separating wave and mean properties which could not be done using time series filtering except below the lowest depth (including both tides and waves) recorded at any time by the vADCP.

142 (3) Low pass filtering using a 4<sup>th</sup> order Butterworth filter with a cutoff frequency of 0.5  
 143 cph to remove all waves and 0.042 Hz to separate infragravity and swell band waves. This  
 144 pair of cutoff frequencies was used with the vADCP data to examine transport variabil-  
 145 ity associated with wave-averaged flows, infragravity waves, and swell.

146 [Figure 1 about here.]

147 [Table 1 about here.]

### 148 3 Observations

#### 149 3.1 Forcing

150 During this study, the forereef tidal range was  $\sim 1$  m (Fig. 2 a), resulting in wa-  
 151 ter depths on the reef flat ranging from  $\sim 35$  cm to 1 m. The wind was weak, with speeds  
 152 less than 5 m/s, and typically toward the West (positive cross- and negative alongshore  
 153 components) (Fig. 2b). The wind-induced surface stress,  $\tau_s$  was estimated with the com-  
 154 monly used quadratic drag law,  $\tau_s = C_D \rho_{air} U_a |U_a|$ , where  $\rho_{air} = 1.23 \text{ kg/m}^3$  is the  
 155 density of air,  $C_D = 0.0008$  is the drag coefficient for wind velocities less than 6.6 m/s  
 156 (Hellerman, 1967), and  $U_a$  is the wind velocity. Thus, on average  $\tau_s \simeq 0.005$  Pa, and  
 157 had a maximum value of ca. 0.033 Pa. Thus, wind stresses were much smaller than the  
 158 other measured forces on the reef flat (see below) and so will be neglected in the rest of  
 159 what follows.

160 [Figure 2 about here.]

161 Figures 2 b and 2 c show: (a) the connection between wave forcing, calculated spec-  
 162 trally as in Monismith et al. (2015) using the pressure data at FR5 corrected for frequency-  
 163 dependent attenuation, and setup between the fore-reef (FR15) and the ocean-ward edge  
 164 of the reef flat (D-5); and, (b) the very small sea surface elevation difference across the  
 165 reef flat (i.e, between D0 and D-5); The setups shown are calculated as the wave-averaged  
 166 depths minus the average depth for the whole record, i.e., for any location

$$\bar{\eta}_* = h - \frac{1}{T_R} \int_0^{T_R} h dt \quad (1)$$

167 where  $h$  is the the measured wave-averaged depth, and  $T_R$  is the length of the record.

168 As will be discussed below, this approach also removes the mean setup, which must be

169 found separately (see also Monismith et al. (2013)). As seen in Fig. 2, there is little cross-  
 170 shore variation in mean sea surface height inshore of breaking; thus, as described by Lindhart  
 171 et al. (2021), the Ofu lagoon appears to be "closed". The dynamics of the reef flat flows  
 172 will be explored further below. Finally, as noted by Kowcek et al. (2015), flows in the  
 173 lagoon are in phase with tidal elevation and are clearly related to the strength of the wave  
 174 forcing. Thus, the Ofu reef might be better described as "mostly closed" in that waves  
 175 do force flows through the lagoon despite the high resistance associated with large rough-  
 176 ness in the lagoon (see Rogers et al. (2018)) and the narrow exit channel between pools  
 177 400 and 500.

178 Wave forcing on the forereef consisted of longer-period (12-22 s) swell events and  
 179 local short period (4-7 s) waves, with  $H_{rms}$  ranging from  $\sim 0.4$ -1 m (Fig. 3), that broke  
 180 normal to the crest ( $-0.8^\circ \pm 2.3^\circ$ ). Applying the approach of Sheremet et al. (2002) (their  
 181 equations 2 and 3) to the FR15 ADP wave burst velocity and pressure data, we found  
 182 that ca. 10 % of the sea-swell (SS) wave energy flux was reflected seaward off the for-  
 183 ereef. Due to this relatively small amount of reflection and the near-normal wave direc-  
 184 tion on the reef, we assume in our analysis below that 100% of the energy flux is shore-  
 185 ward.

186 Offshore of the reef crest, virtually all of the wave energy was in the sea-swell band,  
 187 whereas inshore of breaking, virtually all of the wave energy is in the infra-gravity wave  
 188 band (Maticka, 2019) (see Supplementary material Fig S.1). On the reef flat itself, the  
 189 bore-like waves were: (a) dissipated by bottom friction and continued breaking; and, (b)  
 190 weakened by nonlinear transfers from high to low frequencies. By the end of the reef flat  
 191 the bores had become transformed into trains of nonlinear solitary-like waves, i.e., there  
 192 was rank ordering of waves in each wave train. The details of these transformations are  
 193 discussed in Maticka (2019).

194 [Figure 3 about here.]

195 As is commonly found (e.g. Lowe et al. (2009)), waves on the reef flat varied with  
 196 water level (Fig. 3 a,b), with  $\sim 60\%$  reduction in  $H_{rms}$  from high tide to low tide. Ev-  
 197 idently, for the Ofu reef, the majority of tidal variations in wave height take place on the  
 198 shallow forereef where breaking occurs (Maticka (2019)). Unlike what is seen on beaches  
 199 (e.g. Raubenheimer et al. (1996)), inshore of D-5, local wave height was not a constant

200 fraction of the local depth (Fig. 3 b). This reflects the fact that bottom friction appears  
 201 to be more important than depth-limited wave breaking for dissipating energy on the  
 202 reef flat. Indeed, following Péquignet et al. (2011), we examined modeling dissipation  
 203 due to wave breaking between D-5 and D-3 using the model of Battjes and Janssen (1978);  
 204 unfortunately, dissipation computed using this method was uncorrelated with the dis-  
 205 sipation estimated from the difference in shoreward wave energy flux. In contrast, the  
 206 observed dissipation rate could be fit to the standard bottom friction model,  $\epsilon = 0.6\rho f_w U_{rms}^3$ ,  
 207 with a wave friction factor,  $f_w = 0.34 \pm 0.002$  ( $r^2 = 0.88$ ).

208 In addition to the depth-dependent wave-height reduction, incident gravity wave  
 209 wave energy was transferred to both infragravity and far-infragravity bands (see also Péquignet  
 210 et al. (2014)). Evidently, this infragravity wave forcing excites multiple resonant oscil-  
 211 lations in the lagoon (Fig. 4), presumably at the natural frequencies of the lagoon (Maticka,  
 212 2019). These too vary tidally in both strength and, because of large relative variations  
 213 in lagoon depth, period. We note that the infragravity wave behavior seen on Ofu has  
 214 also been observed on the Ouano barrier reef system (Sous et al., 2019).

215 [Figure 4 about here.]

### 216 3.2 Free Surface Response to Waves

217 The setup or setdown of the free surface relative to offshore,  $\bar{\eta}_r$  represents the wave-  
 218 averaged effect of wave forcing on the free-surface (Longuet-Higgins & Stewart, 1964; Mei  
 219 et al., 1989). As shown by Vetter et al. (2010), this setup depends on the incident wave  
 220 energy flux,  $F$ , the depth on the reef flat,  $h_r$ , and the breaking depth fraction for depth-  
 221 limited breaking,  $\gamma$ , *viz.*,

$$\bar{\eta}_r \simeq \frac{3\gamma^2}{8 + 3\gamma^2} \left( \frac{(8F_w/\rho)^{2/5}}{g^{3/5}\gamma^{4/5}} - h_r \right) \quad (2)$$

222 Thus, per eq.2 the amount of setup may vary with the tides due to tidally modulated  
 223 breaking (Callaghan et al., 2006): As the water level decreases, setup on the reef increases.  
 224 Per eq. 2, there will be a limiting value of  $F$  for a given value of  $h_r$  for which there is  
 225 no breaking and hence no setup. Generally  $\gamma$  depends on the geometry of the reef, i.e.,  
 226 fore-reef steepness (Raubenheimer et al., 1996) and the presence or absence of a reef crest  
 227 ridge (Yao et al., 2012); thus, at present  $\gamma$  should be viewed as a free parameter to be  
 228 determined from observations.

[Figure 5 about here.]

In the present case, we defined  $\bar{\eta}_r$  as the difference in wave-averaged sea level height between the fore-reef and the first reef flat station, i.e.,  $\bar{\eta}_{D-5} - \bar{\eta}_{FR15}$ . Computing setups using eq.1 removes any mean differences in elevation between the pressure sensors, but also removes the mean setup. Thus, there is an offset that must be determined using additional information. Based on eq. 2, the depth of no setup can be estimated by plotting  $\Delta\bar{\eta}_{r*} = \bar{\eta}_{D-5*} - \bar{\eta}_{FR5*}$  as a function of  $F$  and forereef depth. Doing so, we find that the offset between FR5 and D-5 is  $\simeq 12$  cm (see Supplementary material Fig S.2), i.e.,  $\Delta\bar{\eta}_r = \Delta\bar{\eta}_{r*} + 0.12$ . Predicted and observed setup time series using this value, and  $\gamma = 0.87$  and  $h_r$  varying tidally with a minimum depth of 0.25 m are shown in Fig. 5.

Setup calculated on the reef flat (D-5) was generally consistent with predictions made using eq. 2, where  $\bar{\eta}_r$  increases with wave energy flux, and decreases with reef depth as a result of tidally-modulated wave breaking (Fig. 5c). The error in model predictions (figures 5 d and e) was generally small, except near low and high water. Arguably this reflects changes in breaking dynamics; in principle,  $\gamma$  could be made a function of fore-reef depth to better match the observations, but this would not likely have much generality and so would not be particularly useful. Nonetheless, the value of  $\gamma$  found here is similar to that found by Monismith et al. (2013) for the Moorea forereef (0.98).

Setup in the lagoon (D0) followed the same trend, with little difference in free surface elevation between the reef flat and the lagoon (Fig. 2d). This is the behavior that is expected for a closed lagoon (Gourlay, 1996; Lowe et al., 2010), i.e., a nearly spatially-uniform setup in the cross-shore direction. In contrast, the free surface height in an open lagoon will be equal to that of the offshore ocean (Symonds et al., 1995). Evidently, for the case of Ofu, the combination of high friction in the lagoon (Rogers et al., 2018) and the narrowness of the exit channel relative to the overall alongshore length of the lagoon (20 m vs ca. 500 m) combine to create a relatively closed system.

[Figure 6 about here.]

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### 3.3 Flows on the Reef Flat

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While the difference in setup between the reef flat and the lagoon was small, there was flow across and along the reef flat (fig. 6). The cross-reef flows were strongly sheared, with offshore flows near the bottom at high tide early in the record, and over nearly all of the depth at high tides in the later part of the record. Throughout the record, flows were onshore at low tides. The transition between these two conditions took place near when  $\bar{\eta} \simeq 0$ . In contrast to the strongly sheared cross-reef flows, the alongshore flows were nearly unsheared and were primarily directed towards the channel to the northeast.

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The volumetric flux per unit width (transport),  $\bar{q}(t)$  was calculated by integrating the V-ADCP velocity profiles ( $U_x(z, t), U_y(z, t)$ ) over depth using the assumption that the velocity was zero at the bed ( $z = 0$ ). Thus, the wave-averaged flow is given as:

$$(\bar{q}_x, \bar{q}_y) = \overline{\int_0^{h+\eta(t)} (U_x, U_y) dz} \quad (3)$$

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The transport in the lagoon was mostly directed alongshore and had a strong tidal variation, but with a magnitude that was dependent on the strength of the wave forcing on the forereef (Fig. 2e). On the reef flat, the vADCP resolved approximately 90% of instantaneous depth. Thus, because the averaging was applied to the instantaneous transport, the computed wave-averaged flows (shown here for the  $x$  direction) include both the mean Eulerian transport,  $\bar{q}_{x,E}$  and the wave transport,  $\bar{q}_{x,W}$ , i.e., the time-averaged transport due to waves (Monismith et al., 2013). This decomposition conventionally involves writing

$$\bar{q}_{x,E} = \overline{\int_0^{\bar{h}} \bar{U}_x dz} \quad (4)$$

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and

$$\bar{q}_{x,W} = \bar{q}_x - \bar{q}_{x,E} \quad (5)$$

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which is equivalent to averaging the instantaneous wave transport.

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Potential issues with computing the wave transport using the vADCP data are: (1) the relatively low sampling frequency of the vADCP; (2) the fact that depth for the vADCP was measured using surface tracking; and (3) the inability of the instrument to measure velocities in the upper-most 10% of the water column. The low time resolution means that the swell-band waves were not fully resolved, potentially causing the wave transport computed per eq. 5 to be smaller than the actual value. The use of surface track-

284 ing rather than pressure to measure the instantaneous depth likely results in increased  
 285 noise in the surface elevation time series. Lastly, the missing near-surface region is where  
 286 surface rollers form (Svendsen (1984)), and so the vADCP-measured transport likely does  
 287 not include transport associated with the rollers.

288 One way to assess the ability of the vADCP to accurately measure the wave-induced  
 289 velocity is to look at the velocity data in light of shallow water theory which gives the  
 290 instantaneous wave-induced velocity (which is independent of depth),  $\tilde{U}_x$  as a function  
 291 of the instantaneous free-surface deflection  $\tilde{\eta}$  and mean depth,  $h$ ,

$$\tilde{U}_x = \frac{\tilde{\eta}\sqrt{gh}}{h} \quad (6)$$

292 or equivalently

$$\tilde{U}_{x,rms} = \sqrt{\overline{\tilde{U}_x^2}} = \frac{\sqrt{\overline{\tilde{\eta}^2}}\sqrt{gh}}{h} = \frac{\eta_{rms}\sqrt{gh}}{h} \quad (7)$$

293 In terms of transport, eq. 6 implies that

$$\bar{q}_{x,W} = \frac{\eta_{rms}^2\sqrt{gh}}{h} \quad (8)$$

294 Whereas eq. 7 provides a good description of the vADCP velocities, the measured in-  
 295 stantaneous velocities are poorly correlated with and significantly less than theoretical  
 296 values (see Supplementary figure S.2). As a consequence, the computed wave transports  
 297 are significantly less than what would be calculated using 8.

298 The closest instrument to the vADCP that should properly resolve wave motions,  
 299 and thus can be used to test eq. 8, is the ADP at D0. In this case, the observed wave  
 300 transports were 40% of what would be predicted by eq. 8 ( $r^2 = 0.78$ ) As seen in the  
 301 vADCP data, velocities more closely matched theory, with eq. 7 applies with a constant  
 302 of 0.84 rather than 1 ( $r^2 = 0.82$ ). Moreover,  $\eta_{rms}$  at D-4 inferred from surface track-  
 303 ing matches well values of  $\eta_{rms}$  that would be estimated by interpolation of values of  $\eta_{rms}$   
 304 between D-5 and D-3. Thus, while the vADCP surface tracking may have resolved most  
 305 of the surface wave variance, it did not properly resolve the wave transport, possibly an  
 306 effect of noise. Thus, in the absence of any better estimate, we computed the non-roller  
 307 portion of the wave transport at D-4 using measured values of  $\eta_{rms}$  and eq. 7 multiplied  
 308 by 0.4. To include the additional transport associated with the presumably unresolved  
 309 rollers, we supplemented this estimate with an estimate of the roller transport based on  
 310 eq. 2.10 in Svendsen (1984):

$$q_R = 0.9 \frac{H_{rms}^2}{T} \quad (9)$$

311 where  $H_{rms}$  is the rms wave height, and  $T$  is the wave period.

312 [Figure 7 about here.]

313 Figure 7 shows that instantaneous flows associated with the waves on the reef flat  
 314 were roughly an order of magnitude larger than the wave-averaged flows, with swell band  
 315 and infragravity wave band flows being comparable to each other. Despite the fact that  
 316 the instantaneous flows were wave dominated, the portion of the estimated wave trans-  
 317 port on the reef flat described by eq. 8 appears to have been much less than the mean  
 318 Eulerian transport, behavior also reported for broken waves on Moorea by Monismith  
 319 et al. (2013). On the other hand, for much of the record, the roller transport may have  
 320 been significant. Overall, there was a striking reversal of the wave-averaged cross-reef  
 321 Eulerian flow such that during the latter part of the record shown in Fig. 7, flow was  
 322 nearly directed offshore at times. This behavior will be discussed further below in the  
 323 context of the dynamics of flows on the reef flat. While it is strikingly different from what  
 324 has been reported for other barrier reefs, the appearance of an "undertow", i.e. an off-  
 325 shore directed Eulerian mean flow is something that is commonly seen on beaches. Thus,  
 326 as reflected by the behavior of the mean water levels on the reef flat and in the lagoon,  
 327 Ofu appears to be mostly closed, i.e., beach-like. Another distinctive feature of flows on  
 328 the Ofu reef flat is that the along reef flow is comparable to the cross-reef flow such that  
 329 at high tides, flow on the reef flat is directed towards the channel between pools 400 and  
 330 500 rather than either onshore or offshore. As a result, the flow direction alternates be-  
 331 tween primarily cross-shore at low tide and primarily alongshore at high tide.

332 In addition to the vADCP observation of undertow, Lagrangian drifter tracks (see  
 333 Maticka (2019)) show that besides the main channel, offshore flows were also consistently  
 334 present near station H-1, behavior seen in the vADCP data taken there and discussed  
 335 in (Hefner et al., 2019). However, at high tides, offshore flows also appeared near the D-  
 336 transect, behavior that is similar to rip currents that develop on beaches (MacMahan  
 337 et al., 2006) (see fig. 1 in Rogers et al. (2018)). For the Ofu reef, the fact that outflows  
 338 take place at consistent locations other than in the main channel may reflect the way  
 339 small variations in reef crest topography can support relatively stable, but tidally vari-  
 340 able, plan-form variable currents. On the other hand, for low tides, flows were towards  
 341 and out of the channel, conditions seen in other, more "open" systems (Symonds et al.,  
 342 1995; Hench et al., 2008).

343 In summary, the flows we observed at high tide differed from what would be ex-  
 344 pected of the simple 1D model and are consistent with predictions from numerical sim-  
 345 ulations of closed reef-lagoon systems, for which rip currents and along-reef flows might  
 346 be expected (Lowe et al., 2010). Additionally, we observed an undertow, something not  
 347 captured in the simulations of Lowe et al. (2010) since their model did not resolve the  
 348 vertical flow structure. In contrast, the low tide conditions are consistent with the stan-  
 349 dard 1D conceptual model of wave-driven flows, i.e., there is flow shore-ward over the  
 350 reef flat, then along the lagoon towards the channel, and then out the exit channel.

## 351 4 Momentum Balance on the Reef Flat

352 Due to the narrowness of the Ofu reef flat (ca. 100 m) the incident waves, while  
 353 broken, were not fully dissipated by the time they reached the reef flat stations (D-5, D-  
 354 4, D-3). As the water level decreased with the tide, the fraction of waves that broke on  
 355 the forereef increased, which decreased wave height and energy on the reef flat, but in-  
 356 creased the amount of setup on the reef flat. As the water level rose and waves prevailed  
 357 on the flat, the velocity profiles changed from onshore over the depth to onshore near  
 358 the surface and offshore near the bed. To explain the depth-dependent flow dynamics,  
 359 we first consider the depth-integrated momentum balance of the cross-reef flow, and then  
 360 consider the dynamics of the vertical shear in light of the undertow model of Svendsen  
 361 (1984). These analyses will be done for the section of the reef flat between D-5 and D-  
 362 3, using the vADCP measurements at D-4.

### 363 4.1 Depth-integrated momentum balance

364 The cross-shore component of the 1D steady, wave-averaged, depth-integrated mo-  
 365 mentum equation commonly used to describe nearshore flows is (Mei et al., 1989):

$$\frac{d}{dx} \left( \frac{\bar{q}_x^2}{h + \bar{\eta}} \right) = -g(h + \bar{\eta}) \frac{d\bar{\eta}}{dx} - \frac{1}{\rho} \frac{dS_{xx}}{dx} - \frac{1}{\rho} \frac{dR}{dx} - \frac{\tau_b}{\rho} \quad (10)$$

366  $x$  is the principal flow direction (i.e, cross-reef),  $h$  is water depth,  $\bar{\eta}$  is time-averaged free-  
 367 surface height deviation from mean sea level,  $\tau_b$  is the bottom stress,  $S_{xx}$  is radiation stress  
 368 due to waves (calculated spectrally), and  $R$  is the extra contribution to the wave forc-  
 369 ing due to the presence of surface rollers (Svendsen, 1984). From left to right the terms  
 370 in eq. 10 will be referred to as advection (*ADV*), pressure gradient force (*PGF*), radi-  
 371 ation stress gradient (*RSG*), roller force (*RF*), and bottom friction (*BF*). It is impor-

372 tant to note that the flow appearing in Eqn. 10 is the total flow  $q$ , i.e. the flow includ-  
 373 ing both Eulerian and wave transports (Monismith et al., 2013).

374 Advection is often not important in reef-lagoon systems (Sous et al., 2020). For the  
 375 reef flat at Ofu:  $\bar{q} \approx 0.05m^2/s$ ,  $h \approx 0.5m$ ,  $dq/dx \approx 0$  (from 1D continuity),  $dh/dx \approx$   
 376  $-4 \times 10^{-3}m/m$  (see Maticka (2019)), and  $\bar{\eta} \ll h$ . Thus, we estimate:

$$\frac{d}{dx} \left( \frac{\bar{q}_x^2}{h} \right) = \frac{2\bar{q}_x}{h} \frac{d\bar{q}_x}{dx} - \frac{\bar{q}_x^2}{h^2} \frac{dh}{dx} = -\frac{\bar{q}_x^2}{h^2} \frac{dh}{dx} \approx 4 \times 10^{-5} \frac{Pa}{kg/m^3} \quad (11)$$

377 For most of our study period, this is 1 to 2 orders of magnitude smaller than our esti-  
 378 mates for the other forces. As with wind stresses, advective accelerations are neglected  
 379 in what follows, although it may be important in times when the flow transitions to a  
 380 horizontally varying flow.

381 Thus, neglecting advection, we write eq. 10 as

$$-\frac{1}{\rho} \frac{dS_{xx}}{dx} - \frac{1}{\rho} \frac{dR}{dx} = g(h + \bar{\eta}) \frac{d\bar{\eta}}{dx} + \frac{\tau_b}{\rho} \quad (12)$$

382 where then LHS represents the wave forcing, and the RHS represents the possible response.  
 383 For reef flats in open systems, both the RHS and LHS are zero, i.e., the wave forcing is  
 384 nearly zero and the pressure gradient across the reef balances bottom drag (c.f. the Moorea  
 385 reef - Monismith et al. (2013)), whereas for closed systems, e.g. beaches, the wave forc-  
 386 ing is balanced by the pressure gradient, and the flow and thus the bottom drag are small.  
 387 We will consider each of the terms in eq. 12 in turn below.

388 *RSG* ( $-dS_{xx}/dx$ ) is calculated using finite differences as  $-dS_{xx}/dx \simeq -\Delta S_{xx}/\Delta x$   
 389 where  $\Delta x$  is the cross-shore distance from D-5 to D-3.  $\Delta S_{xx}$  was calculated by differ-  
 390 encing the results of spectral integration of the wave data at D-5 and D-3 including both  
 391 the sea & swell and the infragravity frequency bands ( $f_1-f_2=0.004-0.25Hz$ ):

$$S_{xx} = \rho g \int_{f_1}^{f_2} P_{\eta\eta}(f, t) \cdot \left( 2 \frac{C_g}{C} - \frac{1}{2} \right) df \quad (13)$$

392 Here  $f$  is the frequency,  $t$  is time,  $C_g$  is wave group velocity,  $C$  is wave phase speed, and  
 393  $P_{\eta\eta}$  is the spectral density of variations in the free surface height modified to account  
 394 for frequency-dependent attenuation of the pressure. The roller force was calculated fol-  
 395 lowing Fredsøe and Deigard (1992) as:

$$R \simeq 0.9\rho \frac{H_{rms}^2 C}{T} \quad (14)$$

396 where  $C = \sqrt{gh}$ .

397 While the wave forcing can be calculated explicitly, the *PGF* and *BF* terms both  
 398 include parameters that are unknown *a priori*. The free-surface slope on the reef flat shown  
 399 in Fig. 2 does not include whatever mean setup might have existed during our field ex-  
 400 periment. If a quadratic drag law is used, then the drag coefficient,  $C_D$ , must also be  
 401 determined in some fashion. In what follows, we will pursue an iterative approach to es-  
 402 timate both the unknown mean setup and  $C_D$ .

403 The 1D wave-averaged quadratic drag law

$$\bar{\tau}_b = \rho C_D \overline{U_x (U_x^2 + U_y^2)^{1/2}} \quad (15)$$

404 is often used to represent bottom stress (Grant & Madsen, 1979; Lentz et al., 2017), where  
 405  $(U_x, U_y)$  is the depth- averaged Eulerian velocity, and  $C_D$  is the drag coefficient. When  
 406 waves are present, the velocities appearing in eq. 15 include both wave averaged veloc-  
 407 ities and the wave velocities, i.e.  $U_x = \bar{U}_x + \tilde{U}_x$  (Feddersen et al., 2000). Thus, the bot-  
 408 tom drag acting in the x direction will be

$$\bar{\tau}_b = \rho C_D \overline{(\bar{U}_x + \tilde{U}_x) |\bar{V} + \tilde{V}|} \quad (16)$$

409 where  $V = (U_x^2 + U_y^2)^{1/2}$ . If  $\tilde{V}_{rms} \gg \bar{U}_x$ , then  $\bar{\tau}_b \simeq 2\rho C_D \bar{U}_x \tilde{V}_{rms}$  (Wright & Thomp-  
 410 son, 1983).

411 [Figure 8 about here.]

412 For the surface slope, we started with the difference in  $\bar{\eta}_*$  (defined by eq.1) between  
 413 D-2 and D-5 (60 m separation) rather than between D-3 and D-5. The reason for using  
 414 these two stations rather than the pair D-3 and D-5 to estimate  $\frac{d\bar{\eta}}{dx}$  was that the *PGF*  
 415 calculated using D-3 and D-5 (30 m separation) tended to be too noisy. Note that the  
 416 sea level differences are on par with the stated accuracy of the pressure sensors (ca. 1  
 417 cm) but are still somewhat greater than the stated resolution of the sensors (0.2 mm).

418 Estimates of both the setup offset and  $C_D$  were found by trial and error iteration.  
 419 To do this we computed the lack of closure in the momentum balance, i.e., the error  $E$ ,  
 420 as:

$$E = PGF + BF - (RSG + RF) \quad (17)$$

421 This iteration was carried out by first choosing a value of  $C_D$ , and then finding the off-  
 422 set in setup and  $C_D$  that produced a mean value of  $E \simeq 0$ . The iteration proceeded  
 423 by choosing different values of  $C_D$  and repeating this process, with the goal of making

424  $E_{rms}$  as small as possible and so that a linear fit of  $(PGF+D)$  as a function of  $(RSG+$   
 425  $RF)$ , should give a slope  $\simeq 1$ . Following this procedure we found that the setup adjust-  
 426 ment between D-2 and D-5  $\simeq 7.6$  mm upwards, and that  $C_D \simeq 0.031$ . The resulting  
 427 force time series are shown in Fig.8. These parameters resulted in  $(PGF+D) = (1 \pm$   
 428  $0.01)(RSG+RF)$  ( $r^2 = 0.88$ ), and gave a mean error of 0.004 Pa and an rms error of  
 429 0.15 Pa (see Fig. 8e). One key feature of the momentum balance is that waves on the  
 430 reef flat were always important to the drag (Fig. 8c), such that drag was well described  
 431 by the linear model of Wright & Thompson (1983).

432 For the entire experiment (Fig. 8d), at high tides, the  $PGF$  was directed *offshore*  
 433 and nearly balanced the wave forcing  $(RSG+RF)$ , behavior that is typically observed  
 434 in surf zones on beaches (Longuet-Higgins & Stewart, 1964; Symonds et al., 1995), i.e.,  
 435 from the standpoint of the momentum balance, the Ofu reef is closed at high tide. In  
 436 contrast, at low tides, the wave forcing was primarily balanced by bottom friction  $(BF)$ ,  
 437 behavior characteristic of open reef systems, although in some cases (e.g., Hench et al.  
 438 (2008)) the wave forcing is unimportant and instead  $PGF \simeq BF$ . Thus, the Ofu reef  
 439 behaves as either a closed or open system depending on tidal water level. This behav-  
 440 ior can be visualized using the force-balance "phase plane" shown in Lindhart et al. (2021).  
 441 As seen in Fig. 9, for water levels less than mean sea level,  $RSG + RF$  tends to be  $\simeq$   
 442  $BF$ , although unlike what is seen in model results shown in Lindhart et al. (2021), the  
 443  $PGF$  contributes to the force balance even at the lowest water levels. For water levels  
 444 somewhat greater than mean sea level,  $RSG + RF \simeq PGF$ , although in this case the  
 445  $BF$  still plays a small role in the force balance.

446 [Figure 9 about here.]

447 The value of  $C_D$  we found on the reef flat is an order of magnitude larger than what  
 448 is typically found for smoother surfaces like sandy bottoms on the inner shelf, i.e., ca.  
 449 0.003. This is plausible given the small-scale topography of the reef that varied between  
 450 nearly flat to including corals that were ca. 10 cm high (Fig. 10). This value of  $C_D$  is  
 451 within the (wide) range of drag coefficients reported for reefs, which vary from 0.009 to  
 452 0.8 (Rosman & Hench, 2011). Using the law of the wall (Pope, 2000), Lentz et al. (2017)  
 453 showed that in some cases, variations in water depth could explain variability in  $C_D$  based  
 454 on depth-averaged velocities. To test for this possibility, we considered an alternative  
 455 to using a single value of  $C_D$ : Choose  $C_D(t)$  so the momentum balance was satisfied for

456 each time. Following this approach, we found no systematic variation of  $C_D$  with depth.  
 457 Why this might be the case is that the observed velocity profiles (shown below) often  
 458 cannot be described by the law of the wall, a likely effect of the vertical structure of the  
 459 forcing. We tried using the near-bottom cross-reef velocity to parameterize drag, but do-  
 460 ing so resulted in errors that were consistently larger than what was obtained using the  
 461 depth-averaged velocity, and so are not presented here.

462 [Figure 10 about here.]

## 463 4.2 Vertical Structure of Reef Flat Flows

464 The vertical structure of flows in the surfzone is determined by the local force im-  
 465 balance that arises because the  $RSG$  varies with depth, with the majority of the wave  
 466 contribution to momentum flux occurring between the trough and crest (Svendsen, 1984).  
 467 In particular, when waves are actively breaking, there is a surface roller that effectively  
 468 imposes a shear stress on the underlying fluid. In contrast, because it is due to a slop-  
 469 ing free surface, the  $PGF$  is constant over depth and, if it is directed offshore, produces  
 470 an offshore flow near the bottom, i.e., an undertow. In the present case, what is impor-  
 471 tant is that the  $RSG$  covaries with the tidal elevation (Fig. 8), producing a larger im-  
 472 balance between the  $PGF$  and  $RSG$  throughout the water column at high tide, thus lead-  
 473 ing to stronger undertow.

474 The vertical structure of the reef flat flow can be examined using Svendsen's (1984)  
 475 model for the mean Eulerian flow in surfzones. The local momentum equation valid be-  
 476 low the wave trough (his eq. 4.1) reads

$$\frac{\partial}{\partial z} \left( \nu_t \frac{\partial \bar{U}_x}{\partial z} \right) = \frac{\partial \overline{U_{xw}^2}}{\partial x} + g \frac{\partial \bar{\eta}}{\partial x} \quad (18)$$

477 Thus

$$\frac{\partial}{\partial z} \left( \nu_t \frac{\partial \bar{U}_x}{\partial z} \right) = \frac{\partial \overline{U_{xw}^2}}{\partial x} - \frac{1}{\rho h} \frac{\partial S_{xx}}{\partial x} - \frac{\tau_b}{\rho h} \quad (19)$$

478 Note that  $S_{xx}$  includes the roller contribution,  $R$  (eq. 14), as well as what would be com-  
 479 puted using linear theory, i.e., for shallow water waves

$$S_{xx} = \frac{3}{2} \rho g \eta_{rms}^2 + R \quad (20)$$

480 In analyzing our data, we used eq. 13, rather the simpler approximation eq. 20, to cal-  
 481 culate  $S_{xx}$ , although the two sets of values are quite close. Assuming that the waves prop-  
 482 agate in the  $x$  direction, and using shallow water theory to compute  $\overline{U_{xw}^2}$  (eq. 7), then

483 :

$$\frac{\partial}{\partial z} \left( \nu_t \frac{\partial \bar{U}_x}{\partial z} \right) = -\frac{g}{2h} \frac{\partial \eta_{rms}^2}{\partial x} - \frac{1}{\rho h} \frac{\partial R}{\partial x} - \frac{\tau_b}{\rho h} = a(x) - \frac{\tau_b}{\rho h} \quad (21)$$

484 Including the bottom boundary condition on the stress, integration twice with re-  
485 spect to  $z$  gives:

$$\bar{U}_x = \int_0^z \frac{a(x) \xi + (\tau_b/\rho)(1 - \xi/h)}{\nu_t(\xi)} d\xi + c(x) \quad (22)$$

486 The bottom kinematic boundary condition gives:

$$\bar{U}_x(0) = U_0 \Rightarrow c(x) = U_0 \quad (23)$$

487 with  $U_0$  equal to the velocity near the bottom. This will be specified later. A common  
488 model for eddy viscosity in shallow flows is the parabolic distribution

$$\nu_t = \kappa u_* z (1 - z/h) \quad (24)$$

489 where  $u_*$  is the bottom friction velocity, although in the present case, this may not be  
490 the correct velocity scale. As an alternative, we re-write this as

$$\nu_t = \nu_0 (z/h) (1 - z/h) \quad (25)$$

491 where  $\nu_0$  is expected to be a function of the strength of turbulence produced by wave  
492 breaking and by bottom boundary layer turbulence also associated with the waves. Thus

$$\bar{U}_x = -\frac{h}{\nu_0} \frac{\partial F}{\partial x} \int_{\frac{z_0}{h}}^{\sigma} \frac{1}{(1 - \sigma)} d\sigma + \left( \frac{\tau_b h}{\rho \nu_0} \right) \int_{\frac{z_0}{h}}^{\sigma} \frac{d\sigma}{\sigma} = A \log \left( \frac{h - z}{h - z_0} \right) + B \log \left( \frac{z}{z_0} \right) \quad (26)$$

493 where  $\sigma = z/h$  and, to make the integrations finite, we have replaced the lower limit  
494 with the dimensionless roughness length  $\frac{z_0}{h}$ , as is customary for turbulent channel flows.

495 In principle, a similar adjustment would be needed near  $z = h$ . The radiation stress/forcing  
496 imbalance is

$$F = g \frac{\overline{\eta'^2}}{2} + \frac{R}{\rho} \quad (27)$$

497 and the two constants are

$$A = \frac{h}{\nu_0} \frac{\partial F}{\partial x} \quad B = \frac{\tau_b h}{\rho \nu_0} \quad (28)$$

498 Hence, one parameter to be determined by matching theory to observations is  $\nu_0$   
499 and the other is  $U_0$ . In the absence of some form of turbulence closure that explicitly  
500 accounts for wave breaking and wave turbulence interactions, a simpler approach is to  
501 assume that that  $\nu_0$  is the product of suitable velocity and length scales, i.e.,

$$\nu_0 = \alpha U_{rms} h \quad (29)$$

502 where  $U_{rms}$  is the rms wave-induced velocity. By trial and error we found that  $\alpha \simeq 0.5$   
 503 produced velocity profiles that best matched observed vertical shears. For  $U_0$ , there are  
 504 several possibilities. Svendsen (1984) suggests that  $U_0$  could be estimated from bottom  
 505 boundary layer streaming. Alternatively,  $U_0$  could be set equal to 0. A third possibil-  
 506 ity is that  $U_0$  can be determined by requiring that the mean Eulerian transport calcu-  
 507 lated by the model is the same as that observed. Given the limitations of our data, we  
 508 chose this last approach to evaluate  $U_0$ .

509 [Figure 11 about here.]

510 A comparison of theory and observations is shown in Fig. 11 for 4 profiles with the  
 511 strongest flows into the lagoon (a-d) and 4 flows with the strongest undertows (e-h). Ev-  
 512 idently, Svendsen’s theory does a reasonable job in predicting the structure of the flow,  
 513 although in the cases with strong onshore flow, the shear is stronger near the bottom and  
 514 near the surface than theory would predict, suggesting that eq. 25 may not always be  
 515 an appropriate description of breaking wave turbulence on the reef flat. Likewise, the  
 516 interior of the water column is less sheared than the model predicts for the cases with  
 517 strong undertows (i.e., offshore flows), suggesting that eq. 25 underpredicts the eddy vis-  
 518 cosity in those cases. Finally, in nearly all cases, the near surface shear is stronger than  
 519 predicted, behavior that may reflect the fact that on average, the uppermost part of the  
 520 water column resolved by the vADCP may have been in the roller and so the basic model,  
 521 eq. 18, is not strictly applicable there.

522 In summary, the surf zone flow model of Svendsen (1984) supplemented by a sim-  
 523 ple turbulence model provides a plausible description of the structure of flows on the Ofu  
 524 reef flat. However, its predictive power is limited by the necessity of choosing an appro-  
 525 priate bottom boundary condition on the velocity, and by the complexity of turbulent  
 526 flows with energetic broken waves, most notably near the surface in the roller produced  
 527 by breaking. The vADCP measurements we present here were not designed to charac-  
 528 terize the surf zone flow near the water surface or near the bottom, nor did we have avail-  
 529 able turbulence measurements that might enable us to better estimate bottom stresses  
 530 and eddy viscosities. Given that reef flats with broken waves are a common feature of  
 531 reefs and that flows there are important to wave runup and overtopping (Storlazzi et al.,  
 532 2018), future efforts to examine this flow in more detail seem warranted.

## 5 Discussion and Conclusions

The Ofu reef appears to function as a (nearly) closed system at high tide, i.e., the offshore directed pressure gradient ( $PGF$ ) balances the onshore directed wave forcing ( $RSG+RF$ ) whereas at low tide, it appears open in that the wave forcing is balanced by bottom drag ( $BF$ ). At high tide, the Ofu reef flat is similar to surf zones on beaches. In this case, the *offshore* direction of the  $PGF$  reflects the fact that as the depth on the reef flat increased with the tide, the strength of the wave forcing on the reef flat increased, although the wave-forced free-surface setup between the fore-reef and reef flat decreased. Nonetheless, at all times the wave-driven setup in the lagoon and on the reef flat were nearly the same and so the cross-shore pressure gradient on the reef flat was always much smaller than the pressure gradient in the region near the reef crest where waves first break.

The remarkable vertical resolution of the vADCP allowed us to observe how the variation of wave forcing with water level affected the vertical structure of the flow on the reef flat: As first described by Svendsen (1984), the depth variable difference between the RSG and PGF creates a vertically sheared flow with an undertow that reduces the depth-integrated cross-reef onshore transport (Svendsen (1984)). In effect, the flow structure at high tide (Fig. 11e-h) resembles a Poiseuille-Couette flow (Kundu et al. (2008)) with the surface stress directed onshore and the pressure gradient directed offshore. At low tide the combined effects of the vertically variable wave forcing and the  $PGF$  both acting to force fluid onshore; thus, flows at low tide were more like Couette flows (Fig. 11a-d). Using a parabolic eddy viscosity model in which the velocity scale was the rms wave velocity and not the shear velocity defined by the bottom stress, and choosing the bottom velocity boundary condition so as to match the total mean Eulerian transport, the vertical structure model of Svendsen (1984) could be fit well to the observed velocity profiles.

Although not directly measured, bottom stresses also appeared to be dynamically important when  $\bar{q}_{x,E}$  was directed onshore. More importantly, given the vertical structure of the flow, it is hard to gauge the accuracy or generality of the value of  $C_D$  we derived based on closing the momentum balance. Clearly, future studies of reef flat dynamics should include direct measurements of bottom stresses if at all possible.

Wave-driven flows through the Ofu reef system were strongly modulated by the tides, with the strongest flows in the lagoon observed at high tides although even for the low-

565 est tides, there is flow through the system. In contrast, the situation on the reef flat was  
 566 more complicated. First, the cross-shore wave transport due to both shallow water wave  
 567 dynamics and due to transport in the roller was always onshore whereas  $\bar{q}_{x,E}$  recorded  
 568 by the vADCP was directed onshore for part of the record and offshore for part of the  
 569 record. Thus, the wave transport which we could only estimate rather than directly mea-  
 570 sure, was crucial to sustaining onshore flows. Secondly, for higher tides the principal di-  
 571 rection of transport was directed nearly along the reef, rather than across the reef. This  
 572 rotation is consistent with flow behavior seen in numerical simulations by Lowe et al. (2010)  
 573 and Lindhart et al. (2021) for closed lagoon systems; it is not seen in open systems (e.g.,  
 574 John Brewer Reef - (Symonds et al., 1995) or Moorea - (Monismith et al., 2013)). How-  
 575 ever, the presence of outflows on the reef flat near sta. H-1 (the vADCP data shown in  
 576 Hefner et al. (2019)), suggests that for closed systems, shallow outflow channels may de-  
 577 velop where the reef crest is locally lower than adjacent sections. Whether or not these  
 578 depressions could ultimately develop into "full fledged" channels is an open question.

579 Tidal modulation of flows through the Ofu lagoon also affects residence times

$$T_r(t) = L/\bar{U}_r(t), \quad (30)$$

580 i.e., the time a given water parcel spends in the lagoon after being transported onshore  
 581 from the ocean corresponding to high tides. Here  $L \simeq 600$  m is the approximate along-  
 582 shore extent of pool 400, and  $\bar{U}_r$  is a representative velocity for flow through the lagoon,  
 583 in this case the depth- and wave-averaged alongshore velocity at D0. The strength of the  
 584 modulation, as shown in Fig. 2, appears to have been about a factor of 3 for the ratio  
 585 of largest to smallest values of  $T_r$ , as defined by the 5 % and 95% percentiles of  $T_r$ . Strik-  
 586 ingly,  $T_r$  depended more strongly on water depth than on wave forcing strength (Fig.  
 587 12) during our two week experiment. It should be recognized that eq. 30 is only intended  
 588 to represent a scale for the residence time, since in reality, water parcels that enter the  
 589 lagoon near the channel will spend much less time in the system than do those that en-  
 590 ter near the pool 300/400 separation line. Nonetheless, given that drifter observations  
 591 suggest that  $T_r$  was ca. 1 hr at high tide (Maticka, 2019), the estimates given in Fig. 12  
 592 are reasonable approximations to the average behavior of the system.

593 [Figure 12 about here.]

594 Finally, one feature of the flows on reefs like the Ofu reef that deserves more con-  
 595 sideration in future is the presence of relatively strong infra-gravity waves. Low-frequency  
 596 variability of reef flat flows was comparable to swell-band variability, and water level fluc-  
 597 tuations inside the lagoon showed the presence of multiple resonant modes, behavior also  
 598 seen on the reef flat on Ipan, which also appears to be "closed" (Péquignet et al., 2009)).  
 599 This is different from what was observed for flows and water levels on the much more  
 600 open reef found on Moorea (Monismith et al. (2013)), suggesting that active infragrav-  
 601 ity wave fields are also an important characteristic of closed systems.

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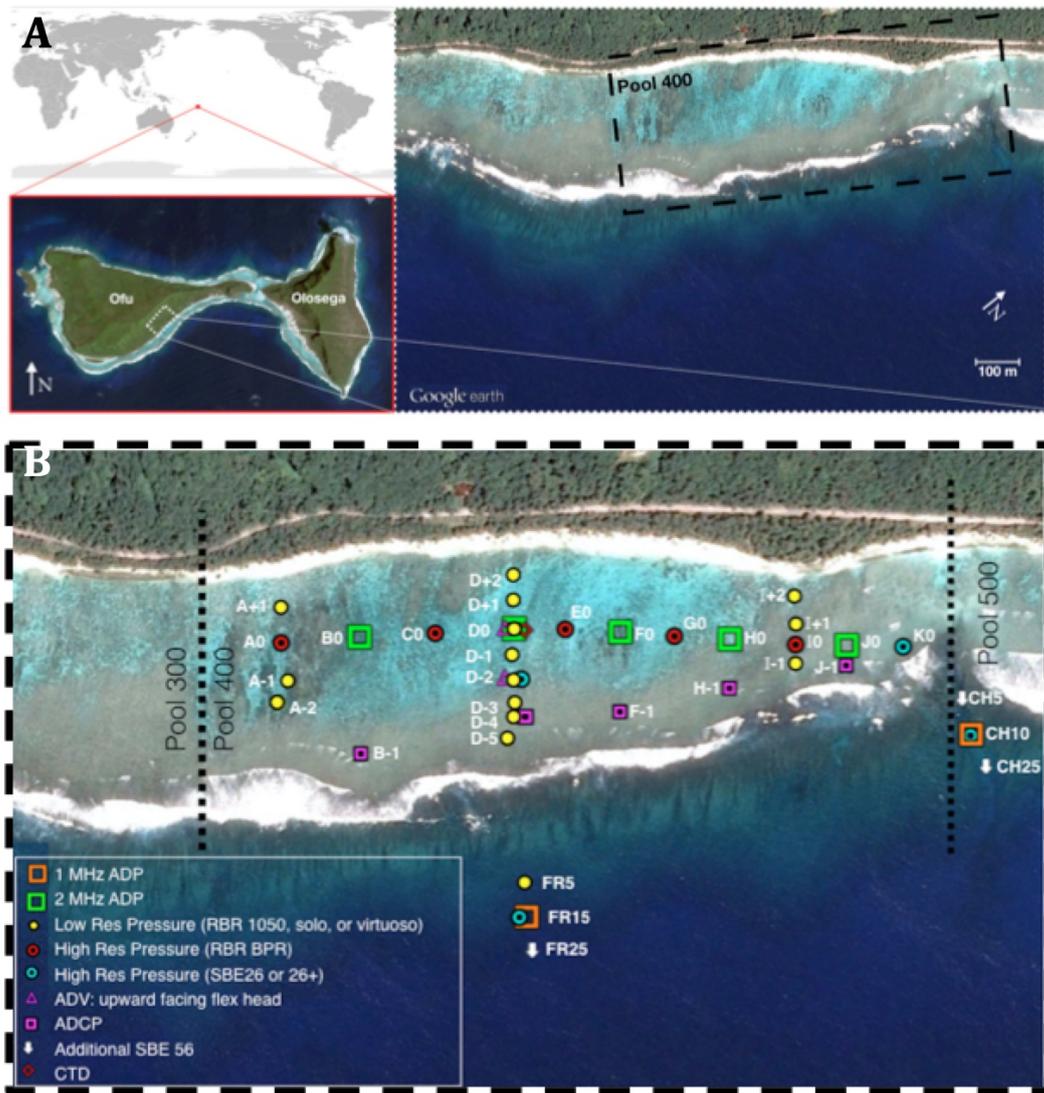
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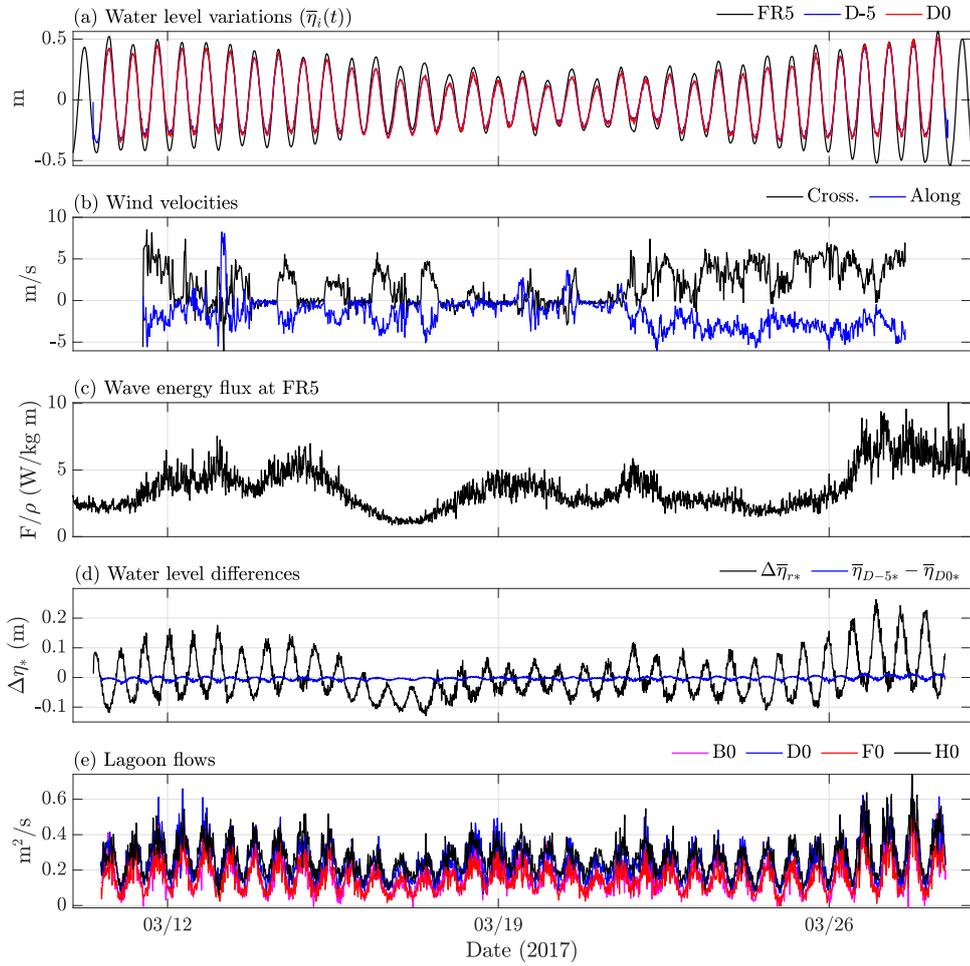
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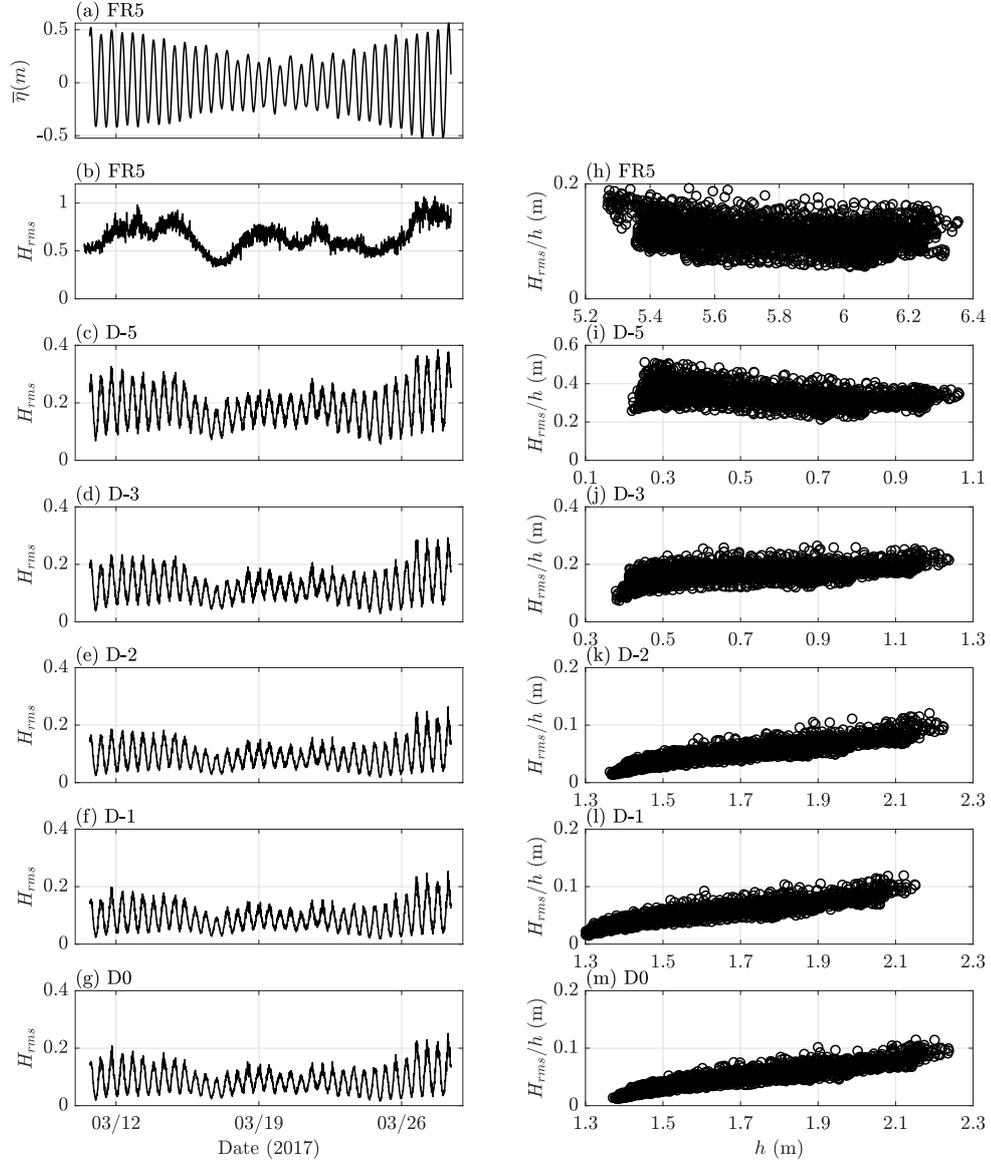
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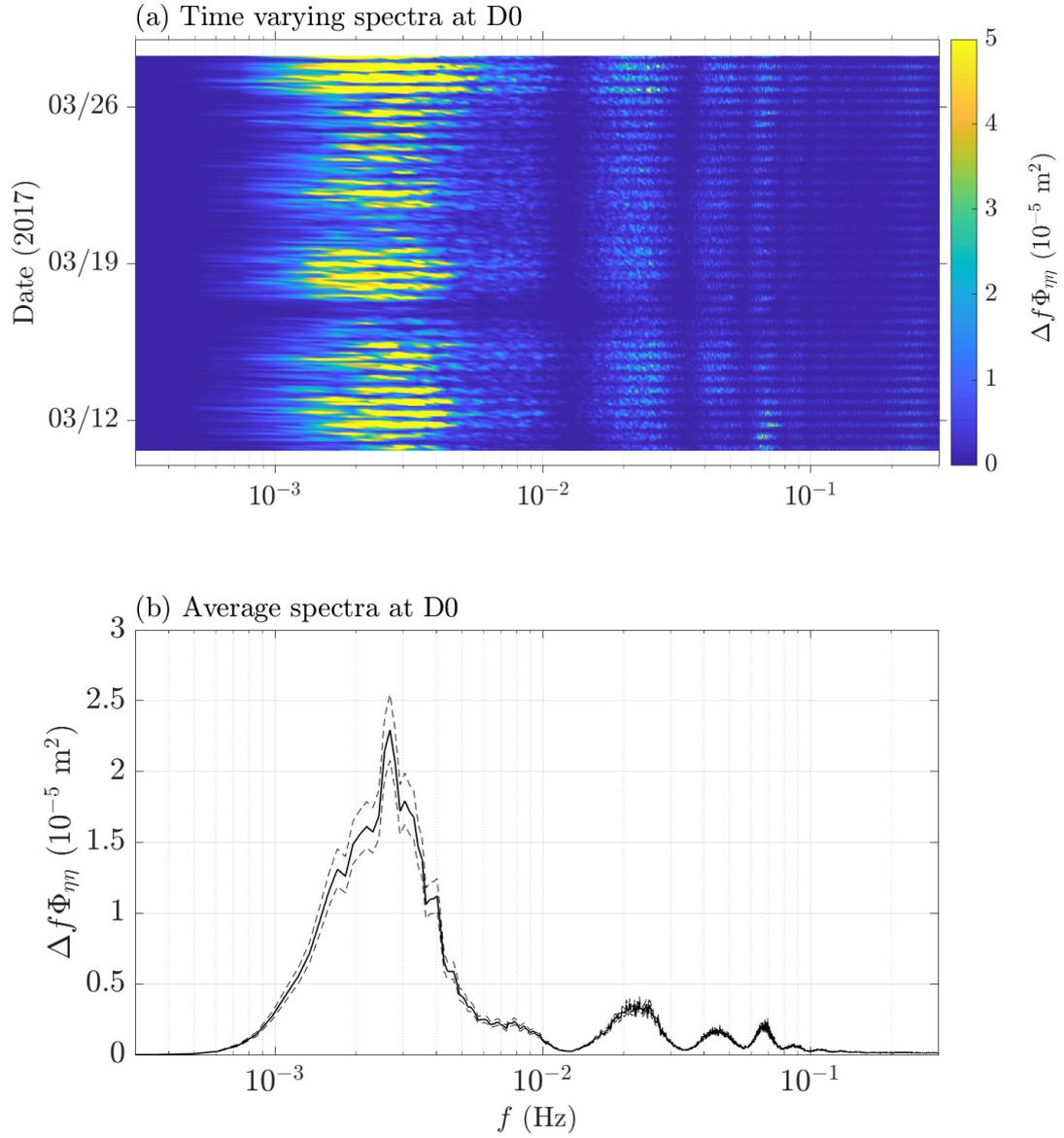
**Figure 1.** a ) Ofu, American Samoa. Pool 400 (dashed box) denotes study focus. b) March 2017 deployment; dashed lines are approximate separation points of pools



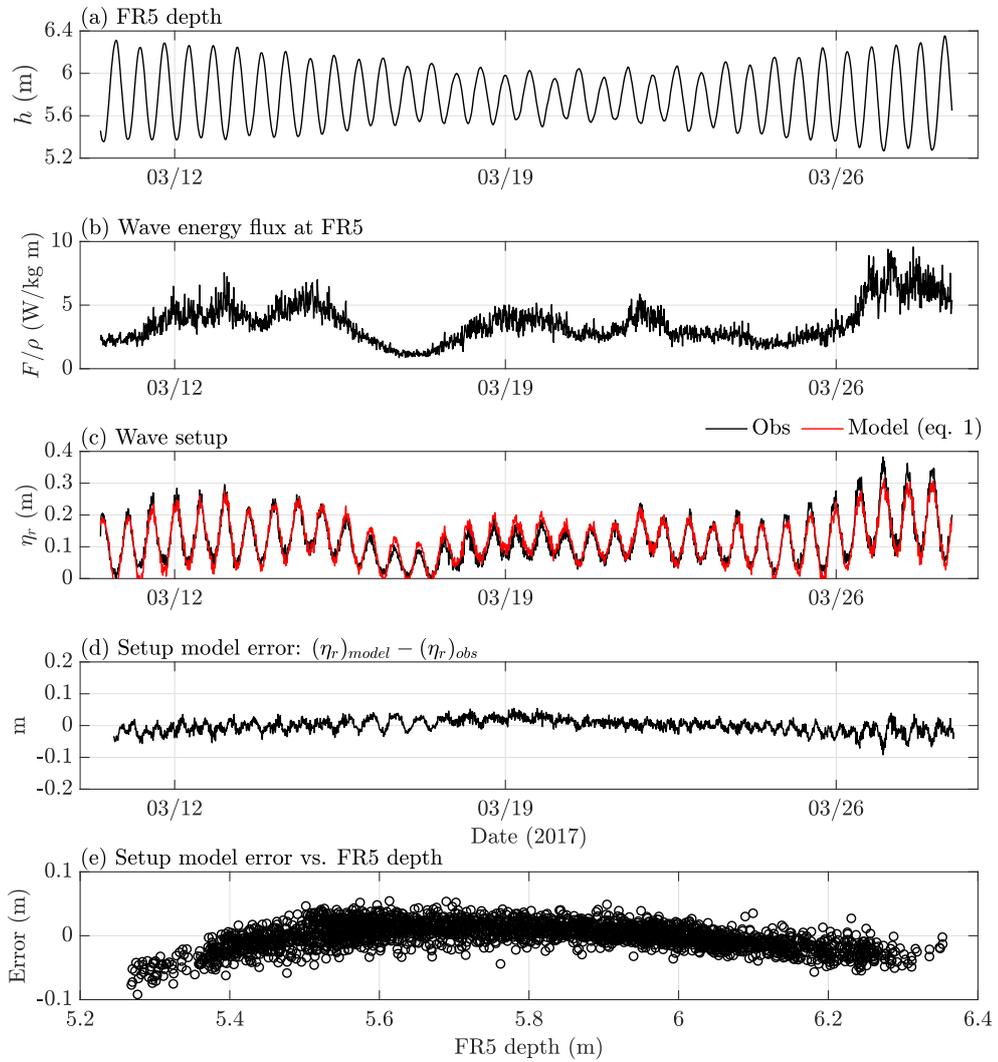
**Figure 2.** Field Conditions during study. a) Water level variations; b) Wind velocities ; c) Wave energy flux on the foreereef (FR5); d) Water level differences fore-reef to reef flat ( $\Delta\bar{\eta}_{r*} = \bar{\eta}_{D-5*} - \bar{\eta}_{FR5*}$ ) and reef flat to lagoon ( $\bar{\eta}_{D0*} - \bar{\eta}_{D-5*}$ ) ; e) Lagoon alongshore transports (positive toward channel)



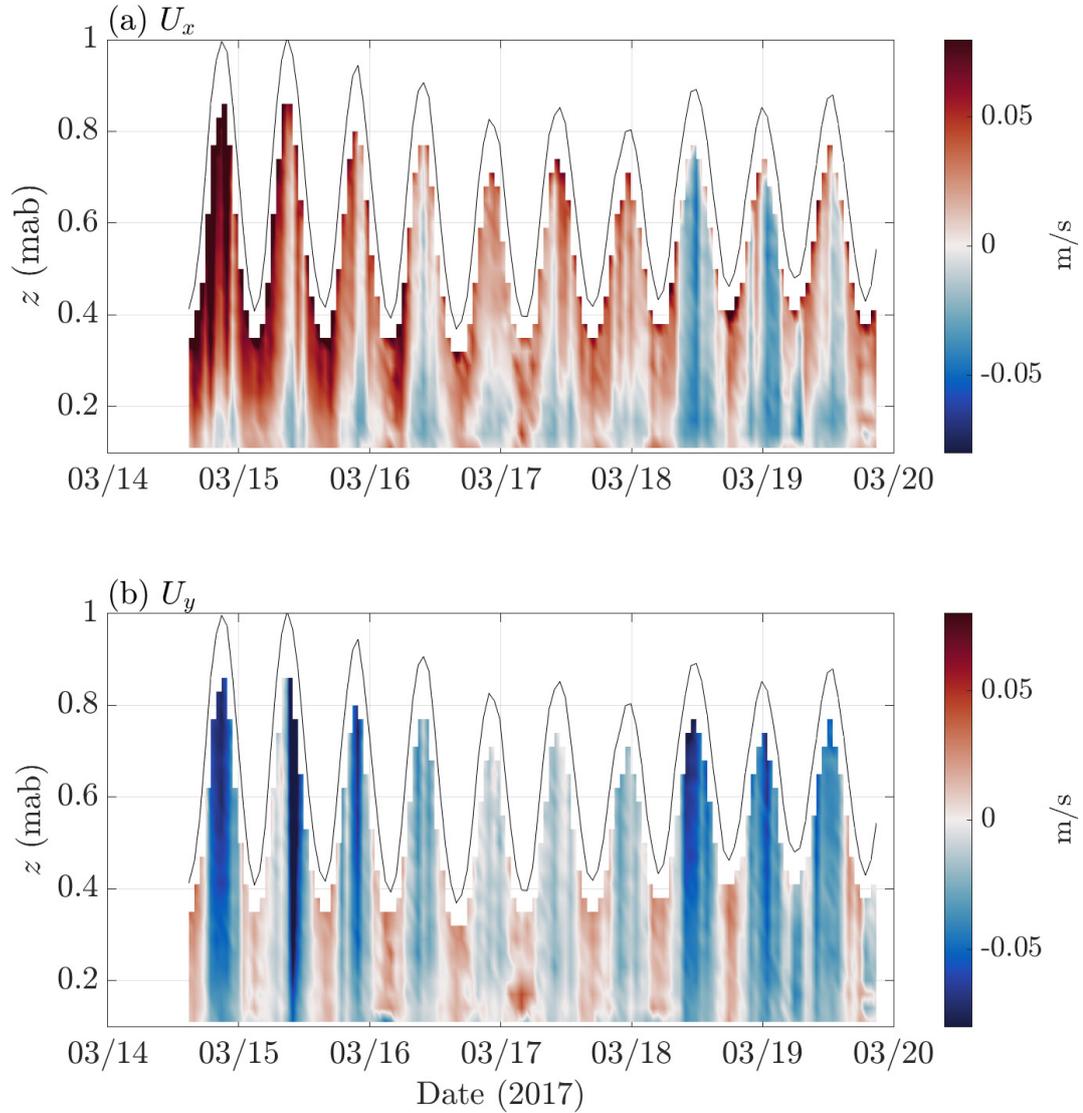
**Figure 3.** (a) Depth at FR5; (b)-(g) RMS wave heights on the D transect line; (h) to (m) RMS wave height for normalized by depth as a function of depth for the same stations shown in (a) to (e).



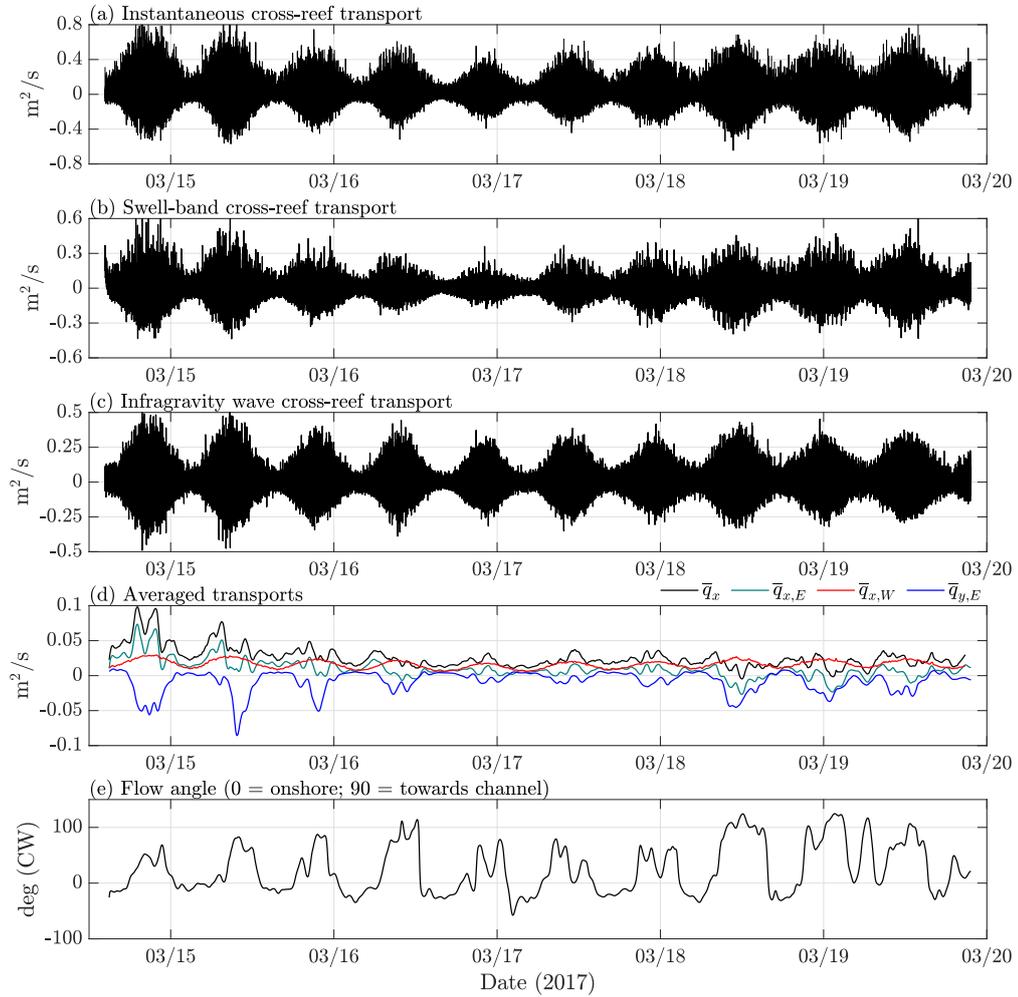
**Figure 4.** (a) Time varying spectra in the Ofu lagoon (D0) (b) Average for entire record of the spectra seen in (a). In (b), the dashed lines show the 95 % confidence intervals.



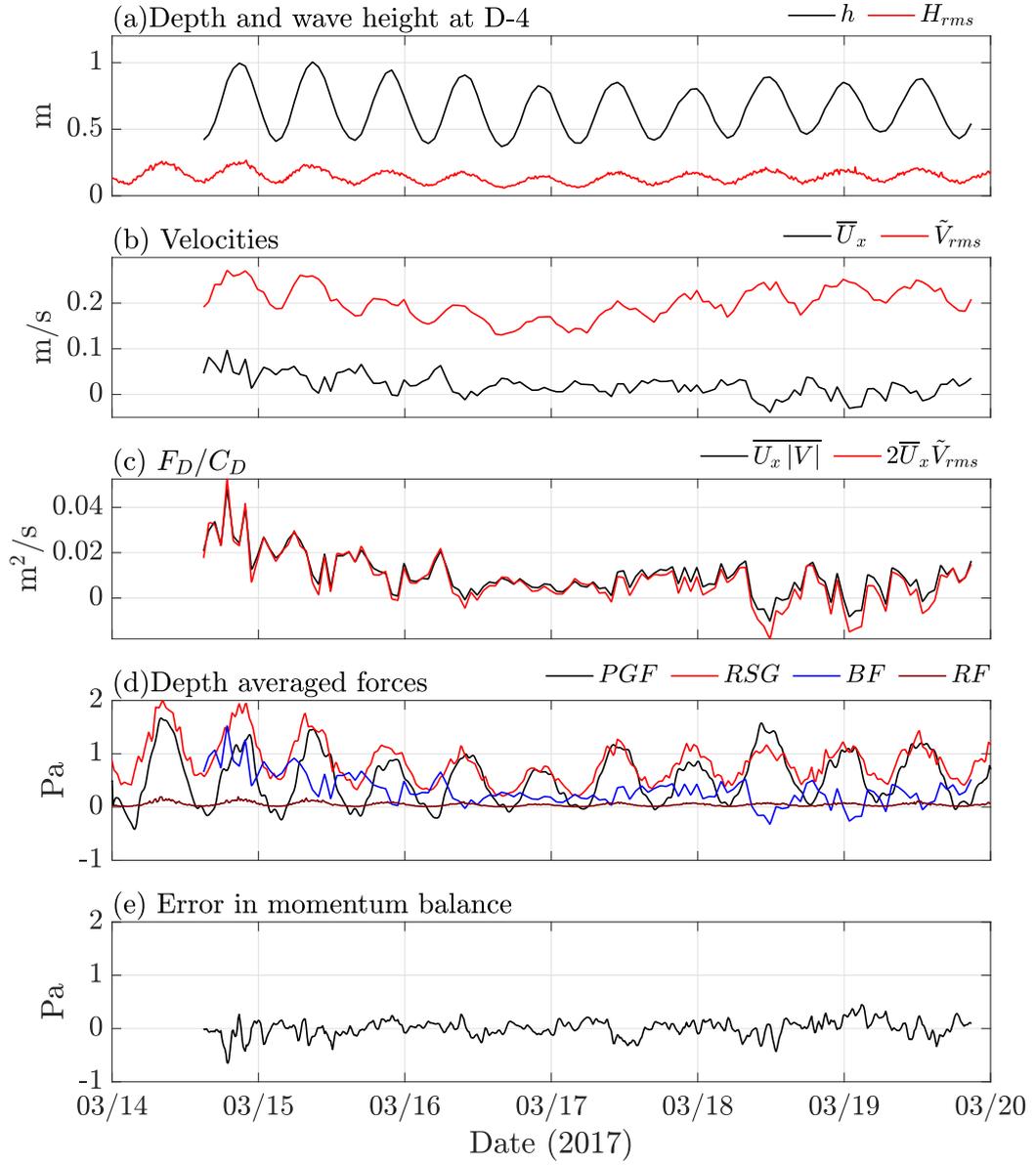
**Figure 5.** (a) Depth on fore-reef (FR5); (b) Wave energy flux at FR5; (c) Observed and modeled setups on reef flat; (d) Error in predicted setup as a function of time; (e) Error in predicted setup as a function of fore-reef depth.



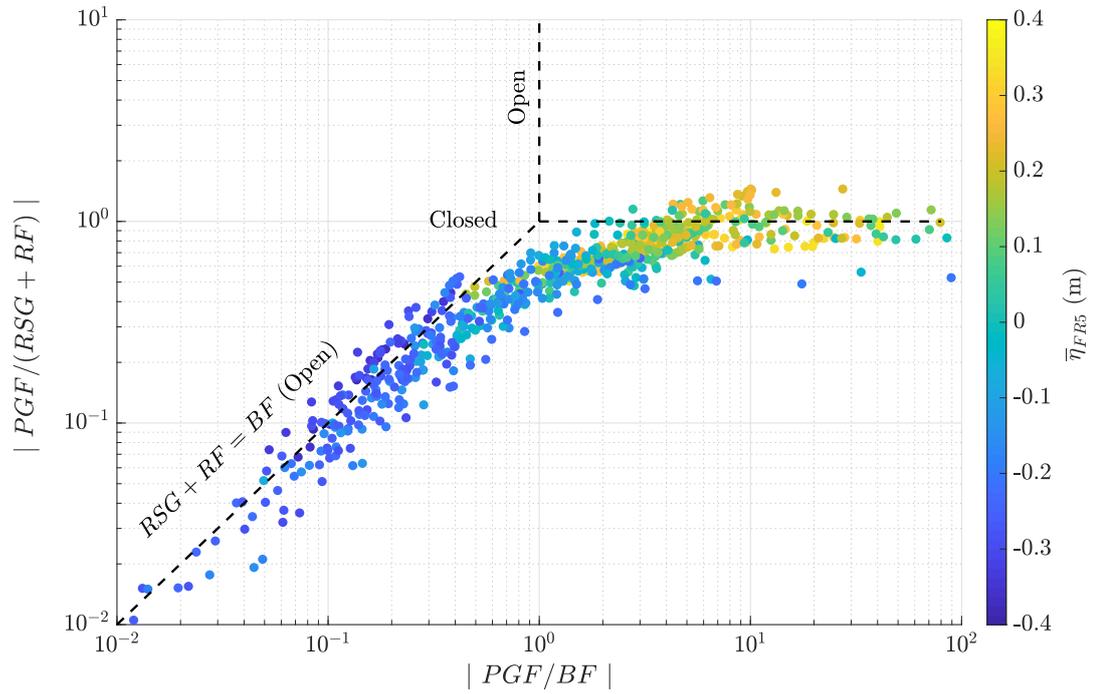
**Figure 6.** vADCP measured velocities at D-4: (a) Cross-reef flows (+ve onshore); (b) Along-reef flows (-ve directed towards channel)



**Figure 7.** vADCP transports measured at D-4: (a)  $q$ ; (b) Instantaneous swell band wave transport; (c) Instantaneous infragravity wave transport; (d) Wave averaged transports in the cross-shore ( $x$ ) and along-shore directions ( $y$ ); (e) angle of the wave-averaged flow relative to the  $x$  direction (+ve CW)



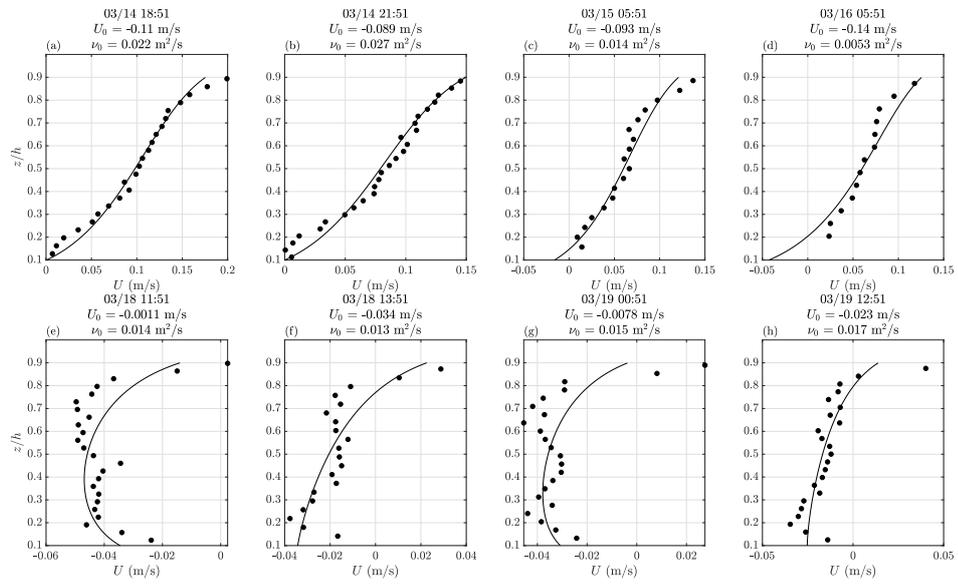
**Figure 8.** Reef flat dynamics: (a) Depth and rms wave height; (b) Wave-averaged cross-reef flow and rms wave velocity; (c) Drag/ $\rho C_D$ : exact and the model of Wright and Thompson (1983); (d) Forces on the reef flat - individual terms are defined in eq.10; (e) Error as given by eq. 17.



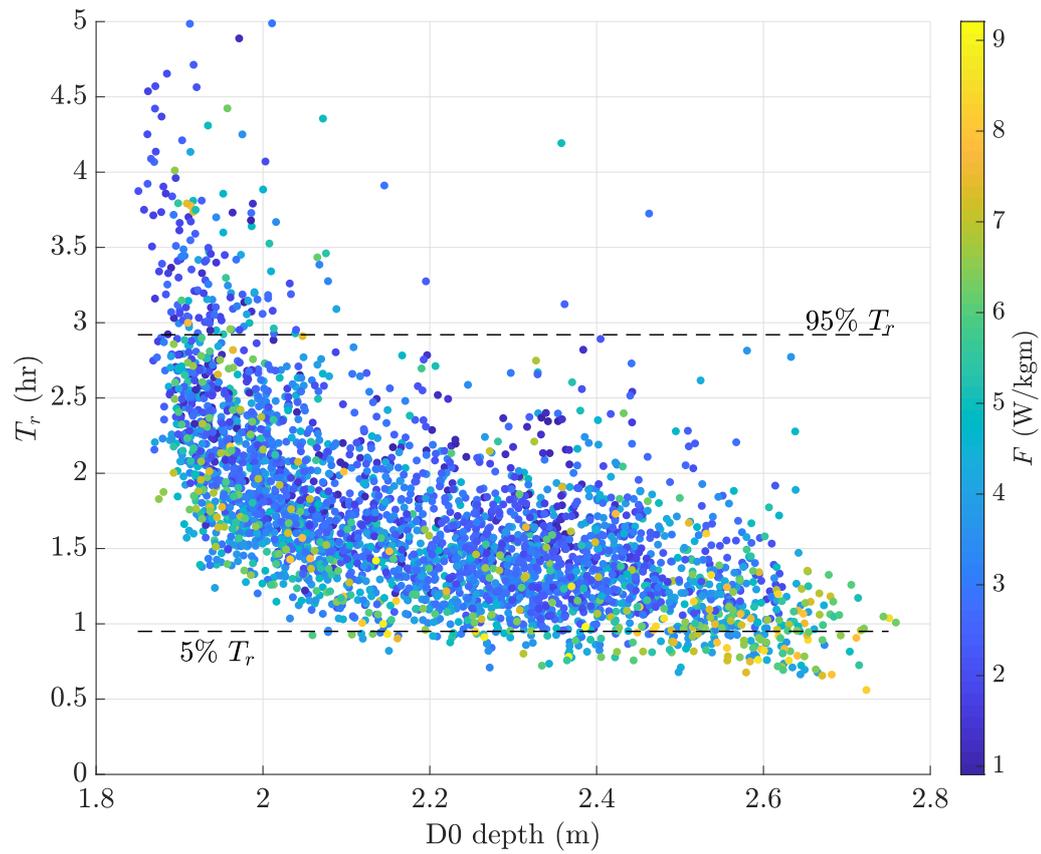
**Figure 9.** Reef flat dynamics: Force balances as a function of depth. The dashed lines indicate different regimes of the force balance defined by eq.10.



**Figure 10.** Sample images of Ofu reef flat showing low relief topography and sparse coral colonies. The largest features in these photos are roughly 10 cm high.



**Figure 11.** Vertical structure of flows on the reef flat dynamics: (●) observations and (—) Svendsen's (1984) model. In each case, times, and values of  $U_0$  and  $\nu_0$  are given.



**Figure 12.** Residence time scale estimated from the velocity measured at D0. The two dashed lines indicate the 5% and 95% residence times based on the entire record.

Station	Location	Depth (m)	Instruments	Sampling
D0	Lagoon	1.72	RBR Solo Pressure	2 Hz continuous
			2 MHz Nortek ADP	Profile: 3' intervals; 0.15 m bins Waves: Burst 30'; 1024 samples @ 2 Hz
D-1, D-2, D-3, D-5	Reef flat	1.6, 1.4, 0.72, 0.55	RBR Solo Pressure	2 Hz continuous
D-4	Reef flat	0.65	TRDI vADCP	0.33 Hz; 0.03 m bins; 1st bin: 0.11 mab
FR5	Forereef	5.8	RBR Solo Pressure	2 Hz continuous
FR15	Forereef	15.4	Seabird SBE26+ Pressure	Tides: 10' (1' avg.); Waves: Burst 30'; 1024 samples @ 2Hz
			1 MHz Nortek ADP	Profiles: 5' intervals; 0.5 m bins Waves: Burst 30'; 1024 samples @ 1 Hz
H-1	Reef flat	0.59	TRDI vADCP	0.33 Hz; 0.03 m bins 1st bin: 0.11 mab

**Table 1.** Wave and flow measurements Ofu March 2017. Instruments shown are ones referred to in this paper. Details for the complete set of instruments shown in Fig. 1 can be found in Matlicka (2019)